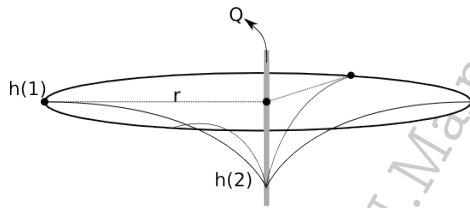


1 Pumping Tests

Pumping Tests

- Large scale tests
 - Single well is pumped
 - one (or more) observation wells used to measure aquifer response
 - shape of cone of depression used to determine K and S_s
- time-drawdown tests
- distance-drawdown tests

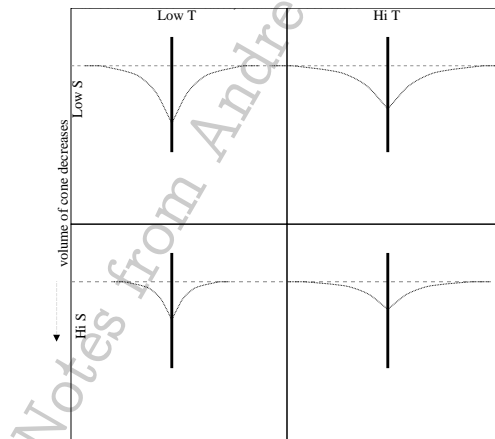


Cone of Depression

Question?

How will the shape of a cone of depression change in response to different values of T and S ?

Cone of Depression



Type-Curve Matching

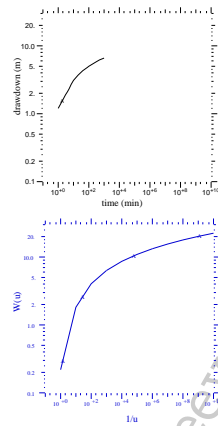
- Use Theis solution to flow equation

$$h_0 - h_{r,t} = \frac{Q}{4\pi T} \int_u^\infty \frac{e^{-u}}{u} du$$

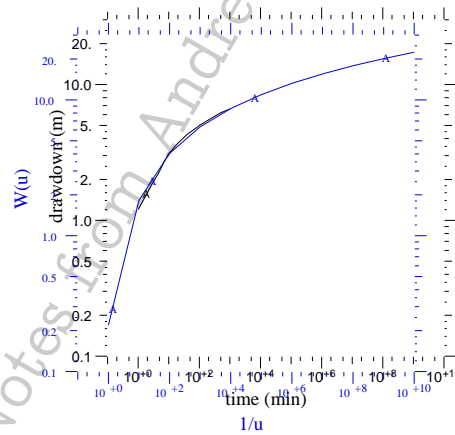
$$u = \frac{r^2 S}{4Tt}$$

- make log-log plot of $(1/u)$ vs $W(u)$
- make log-log plot of time vs drawdown
- match data to type curve
- pick common point, find $(1/u), W(u), t$, and $h - h_0$ at point
- solve above equations for T and S

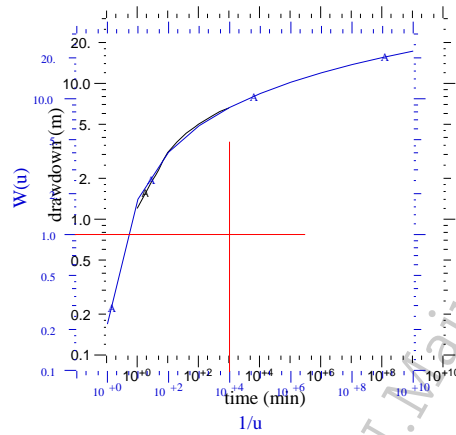
Type-Curve Matching



Type-Curve Matching



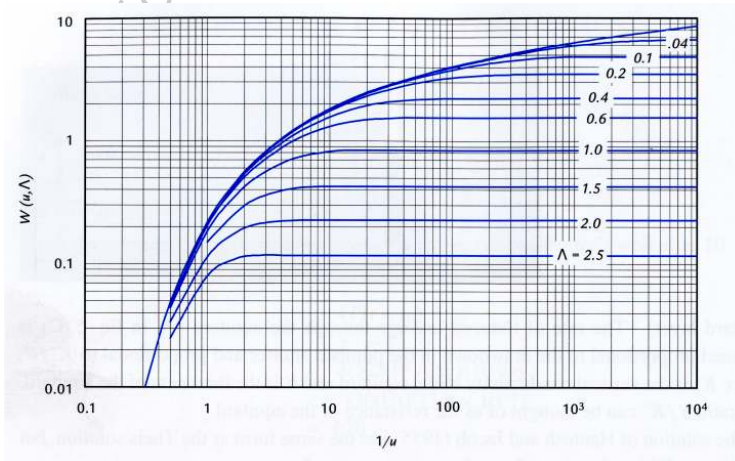
Type-Curve Matching



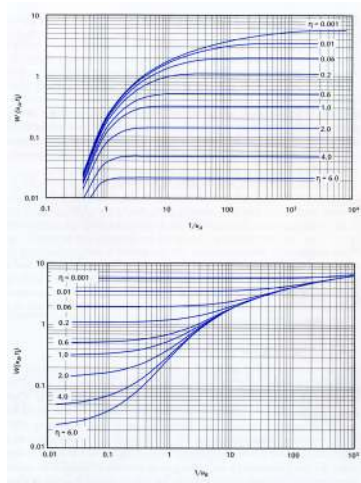
Type-Curve Matching

- Scales on data graph and type-curve graph must be identical
- When matching graphs, axes must be kept parallel
- Different type-curves available for different assumed conditions
 - leaky confining unit
 - unconfined flow

Type-Curve Matching



Type-Curve Matching



Cooper-Jacob Straight-Line Method

$$h_0 - h = \frac{Q}{4\pi T} W(u)$$

$$W(u) = -.5772 - \ln(u) + u - \frac{u^2}{2 \cdot 2} + \dots$$

$$h_0 - h = \frac{Q}{4\pi T} (-.5772 - \ln(u))$$

$$h_0 - h = \frac{Q}{4\pi T} (\ln\left(\frac{1}{u}\right) - .5772)$$

$$h_0 - h = \frac{Q}{4\pi T} (\ln\left(\frac{1}{u}\right) - \ln(1.781))$$

$$h_0 - h = \frac{Q}{4\pi T} \ln\left(\frac{1}{1.781u}\right)$$

$$\ln(u) = 2.3 \log(u)$$

$$u = \frac{r^2 S}{4Tt}$$

$$h_0 - h = \frac{2.3Q}{4\pi T} \log\left(\frac{1}{1.781u}\right)$$

$$h_0 - h = \frac{2.3Q}{4\pi T} \log\left(\frac{4Tt}{1.781r^2 S}\right)$$

$$h_0 - h = \frac{2.3Q}{4\pi T} \log\left(\frac{2.25Tt}{r^2 S}\right)$$

Cooper-Jacob Straight-Line Method

- 10-fold reduction in t (or $\frac{1}{r^2}$) = change in drawdown over 1 log cycle.

$$\Delta h_{\log \text{ cycle}} = \frac{2.3Q}{4\pi T}$$

- when drawdown equals zero.

$$0 = \log\left(\frac{2.25Tt_0}{r^2 S}\right)$$

$$1 = \frac{2.25Tt_0}{r^2 S}$$

Partially Penetrating Wells

- Assumed fully penetrating pumping well (horizontal flow)
- Partial penetration induces vertical flow
 - reduced influence in stratified systems ($K_H > K_V$)
 - decreases with distance from pumping well
 - negligible when:

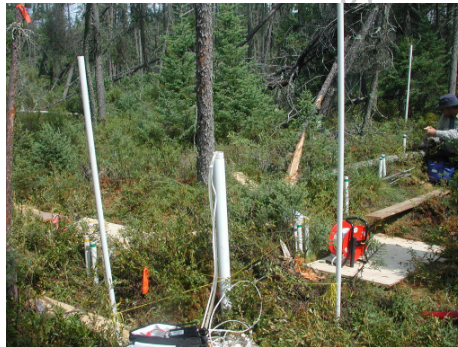
$$r > 1.5 \cdot b \cdot \sqrt{\frac{K_H}{K_V}}$$

- Addition drawdown (Δs) occurs near pumping well ($p = \frac{l}{b}$, b =thickness, l =screen length, $a=1$ (screen at top of aquifer) or 2 (screen centered in aquifer)).

$$\Delta s = \frac{Q}{2\pi T} \frac{1-p}{p} \ln \left(\frac{(1-p) \cdot l}{a \cdot r_s} \right)$$

Planning a Pumping Test

- Need estimate of drawdown vs. Q in pumping well
- Measurement of observation wells
- Monitoring method for pumping rate



Planning a Pumping Test

Discharge Rates:

- Orifice weir
- Orifice bucket
- Open channel Weir or flume

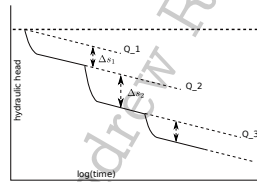


Planning a Pumping Test

- Step drawdown pumping tests
 - estimate T and S
 - estimate drawdown due to different pumping rates
 - determine well loss constant
 - * wells not 100% efficient
 - * more drawdown in well: $s_{real} = s_{theory} + s_{well,loss}$
- increase Q in increments and measure response

Analysis of step drawdown pumping test

- $s_{well,loss} = CQ^2$
- well loss increases disproportionately with Q
- C is a constant determined from step drawdown test
- $C = \frac{\frac{\Delta s_2}{\Delta Q_2} - \frac{\Delta s_1}{\Delta Q_1}}{\Delta Q_2 + \Delta Q_1}$
- $\frac{s_w}{Q} = B + CQ$, B is coef. of loss due to aquifer, C is coef. of loss due to well inefficiency. A linear equation with B as the intercept and C as the slope.



Other Well Jargon

- well efficiency - ratio of theoretical to observed drawdown
- $W.E. = \frac{s_{theory}}{s_{well}}$
- specific capacity - ability of well to deliver water
- $S.C. = \frac{Q}{s_{well}}$
- specific capacity constant if well 100% efficient

Boundaries and Pumping Tests

- Assumed aquifers extend to infinity

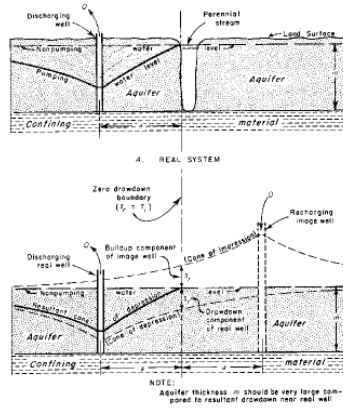
Question?

How will constant hydraulic head and no flow boundaries influence the cone of depression during a pumping test?

Boundaries and Pumping Tests

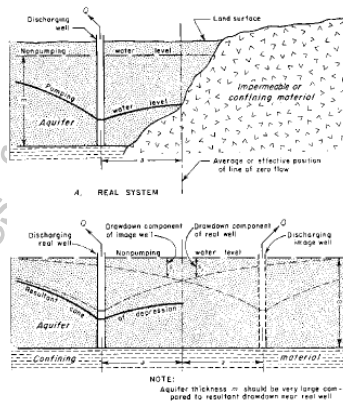
- No flow boundaries
 - increased drawdown
 - must be zero hydraulic gradient oriented normal to the boundary
- Constant head boundaries
 - decreased drawdown
 - head must be unchanged at the boundary
- Image wells used to enforce boundary conditions

Boundaries and Pumping Tests



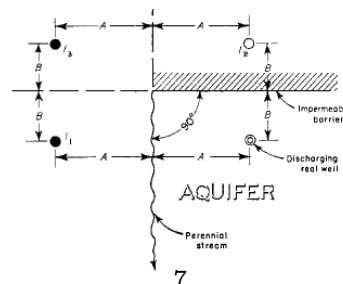
J. G. Ferris, D. B. Knowles, R. H. Brown, and R. W. Stallman. 1962. Theory of Aquifer Tests, Geological Survey Water-Supply Paper 1536-B

Boundaries and Pumping Tests

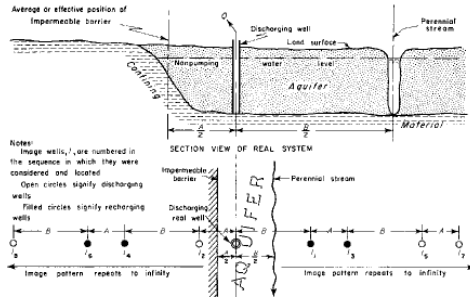


J. G. Ferris, D. B. Knowles, R. H. Brown, and R. W. Stallman. 1962. Theory of Aquifer Tests, Geological Survey Water-Supply Paper 1536-B

Boundaries and Pumping Tests



Boundaries and Pumping Tests



Tracer testing

- velocity from travel time
- point dilution method
 - instantaneously completely mix tracer in well
 - measure conc. change with time
 - groundwater flux dilutes tracer in well

$$\frac{dC}{dt} = \frac{-A \cdot v_{bore} \cdot C}{V_{well}}$$

$$v_{bore} = \frac{-V_{well}}{A \cdot t} \ln\left(\frac{C}{C_0}\right)$$

$$v_{gw} = \frac{v_{bore}}{n_e \cdot \alpha}$$

α is geometry factor that accounts for convergence and divergence of flow lines near well screen, from .5 to 4 in sand

